

PROJECT: 17BP.12.R.78

REFERENCE:

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4	PROFILE
8	CROSS SECTIONS
9-18	BORE LOGS, CORE REPORTS, & CORE PHOTOGRAPHS
19	SITE PHOTOGRAPHS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

STRUCTURE

SUBSURFACE INVESTIGATION

COUNTY IREDELL

PROJECT DESCRIPTION BRIDGE NO. 480254 OVER THIRD CREEK ON SR 1512

SITE DESCRIPTION STRUCTURE AT -L- STATION 13 + 72.50

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	17BP.12.R.78	1	20

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF PREPARING THE SCOPE OF WORK TO BE INCLUDED IN THE REQUEST FOR PROPOSAL. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

SOIL AND ROCK BOUNDARIES WITHIN A BOREHOLE ARE BASED ON GEOTECHNICAL INTERPRETATION UNLESS ENCOUNTERED IN A SAMPLE. INTERPRETED BOUNDARIES MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN SAMPLED STRATA AND BOREHOLE INFORMATION MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

P.M. WEAVER

P. BARRERA

TRIGON EXPLORATION

INVESTIGATED BY ESP Associates, Inc.

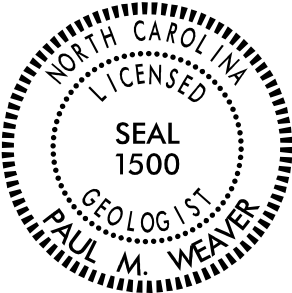
DRAWN BY C.R. PASTRANA

CHECKED BY P.M. WEAVER

SUBMITTED BY ESP Associates, Inc.

DATE September 2021

ESP ASSOCIATES, INC.  
7011 ALBERT PICK RD  
SUITE E  
GREENSBORO, NC 27409  
FIRM # C-0587  
WWW.ESPASSOCIATES.COM



SIGNATUREDATE

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)										SILT-CLAY MATERIALS (> 35% PASSING #200)										ORGANIC MATERIALS									
GROUP CLASS.	A-1		A-3		A-2		A-2-4		A-2-5		A-2-6		A-2-7		A-4		A-5		A-6		A-7		A-1, A-2		A-3		A-4, A-5		A-6, A-7	
SYMBOL																														
% PASSING	50 MX		30 MX		25 MX		10 MX		5 MN		35 MX		35 MX		35 MX		35 MX		35 MX		35 MN		36 MN		36 MN		36 MN		36 MN	
MATERIAL PASSING #40	LL		PI		—		6 MX		—		NP		10 MX		10 MX		41 MN		41 MN		41 MN		41 MN		41 MN		41 MN		41 MN	
GROUP INDEX	0		0		0		0		4 MX		8 MX		12 MX		16 MX		NO MX													
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. OF GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS																			
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD										FAIR TO POOR										FAIR TO POOR		POOR		UNSUITABLE					

PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.75	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)
-----------------	---------------	--------------	------------------------	-------------------	------------	------------

GRAIN SIZE

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL PLASTIC RANGE (PI) PL	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
OM SHRINKAGE LIMIT SL	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

NON PLASTIC	PLASTICITY INDEX (PI)	DRY STRENGTH
SLIGHTLY PLASTIC	0-5	VERY LOW
MODERATELY PLASTIC	6-15	SLIGHT
HIGHLY PLASTIC	16-25	MEDIUM
	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	MODERATELY COMPRESSIBLE	HIGHLY COMPRESSIBLE
LL < 31	LL = 31 - 50	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING

STATIC WATER LEVEL AFTER 24 HOURS

PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA

SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION

SOIL SYMBOL

ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT

INFERRED SOIL BOUNDARY

INFERRED ROCK LINE

ALLUVIAL SOIL BOUNDARY

DIP & DIP DIRECTION OF ROCK STRUCTURES

TEST BORING

AUGER BORING

CORE BORING

MONITORING WELL

PIEZOMETER INSTALLATION

SLOPE INDICATOR INSTALLATION

CONE PENETROMETER TEST

SOUNDING ROD

TEST BORING WITH CORE

SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT

SHALLOW UNDERCUT

UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE

UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK

UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL  
BT - BORING TERMINATED  
CL - CLAY  
CPT - CONE PENETRATION TEST  
CSE - COARSE  
DMT - DILATOMETER TEST  
DPT - DYNAMIC PENETRATION TEST  
e - VOID RATIO  
F - FINE  
FOSS - FOSSILIFEROUS  
FRAC - FRACTURED, FRACTURES  
FRAGS - FRAGMENTS  
HL - HIGHLY

MED. - MEDIUM  
MICA - MICACEOUS  
MOD. - MODERATELY  
NP - NON PLASTIC  
ORG. - ORGANIC  
PMT - PRESSUREMETER TEST  
SAP. - SAPROLITIC  
SD. - SAND, SANDY  
SL. - SILT, SILTY  
SLI. - SLIGHTLY  
TCR - TRI-CONE REFUSAL  
w - MOISTURE CONTENT  
V - VERY

VST - VANE SHEAR TEST  
WEA. - WEATHERED  
% - UNIT WEIGHT  
% - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS  
S - BULK  
SS - SPLIT SPOON  
ST - SHELBY TUBE  
RS - ROCK  
RT - RECOMPACTED TRIAXIAL  
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:  
☐ CME-45C  
☒ CME-55  
☐ CME-550  
☐ VANE SHEAR TEST  
☐ PORTABLE HOIST  
☐ \_\_\_\_\_  
☐ \_\_\_\_\_

ADVANCING TOOLS:  
☐ CLAY BITS  
☐ 6" CONTINUOUS FLIGHT AUGER  
☒ 8" HOLLOW AUGERS  
☐ HARD FACED FINGER BITS  
☐ TUNG-CARBIDE INSERTS  
☒ CASING ☐ W/ ADVANCER  
☒ TRICONE 2 15/16" STEEL TEETH  
☐ TRICONE \_\_\_\_\_ TUNG-CARB.  
☒ CORE BIT  
☐ \_\_\_\_\_

HAMMER TYPE:  
☒ AUTOMATIC ☐ MANUAL  
  
CORE SIZE:  
☐ -B ☐ -H \_\_\_\_\_  
☒ -N NO \_\_\_\_\_  
  
HAND TOOLS:  
☐ POST HOLE DIGGER  
☐ HAND AUGER  
☐ SOUNDING ROD  
☐ VANE SHEAR TEST  
☐ \_\_\_\_\_

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)

CRYSTALLINE ROCK (CR)

NON-CRYSTALLINE ROCK (NCR)

COASTAL PLAIN SEDIMENTARY ROCK (CP)

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.

FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH  
ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.

VERY SLIGHT (V SL.)  
ROCK GENERALLY FRESH, JOINTS STAINED. SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.

SLIGHT (SL.)  
ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.

MODERATE (MOD.)  
SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.

MODERATELY SEVERE (MOD. SEV.)  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*

SEVERE (SEV.)  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*

VERY SEVERE (V SEV.)  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*

COMPLETE  
ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD  
CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.

HARD  
CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.

MODERATELY HARD  
CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.

MEDIUM HARD  
CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.

SOFT  
CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.

VERY SOFT  
CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE  
RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.

MODERATELY INDURATED  
GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.

INDURATED  
GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.

EXTREMELY INDURATED  
SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUPSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BL-2; STA. 14+35.52, 18.30 RT, N 752841.4778, E 1414456.0700

ELEVATION: 863.75 FEET

NOTES:  
F.I.A.D = FILLED IMMEDIATELY AFTER DRILLING

DATE: 8-15-14

# GEOTECHNICAL ENGINEERING UNIT

## ***SUBSURFACE INVESTIGATION***

**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES  
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)					
STRUCTURE	SURFACE CONDITIONS	DECREASING SURFACE QUALITY					COMPOSITION AND STRUCTURE
		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.		From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.					
DECREASING INTERLOCKING OF ROCK PIECES	SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)	DECREASING SURFACE QUALITY					COMPOSITION AND STRUCTURE
		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	
		From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.					
		From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.					
		From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.					
		From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.					
		From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.					



- A

ROADWAY EMBANKMENT: Orange and Brown, Soft, Fine Sandy SILT (A-4), Little Mica and Gravel, Moist
- B

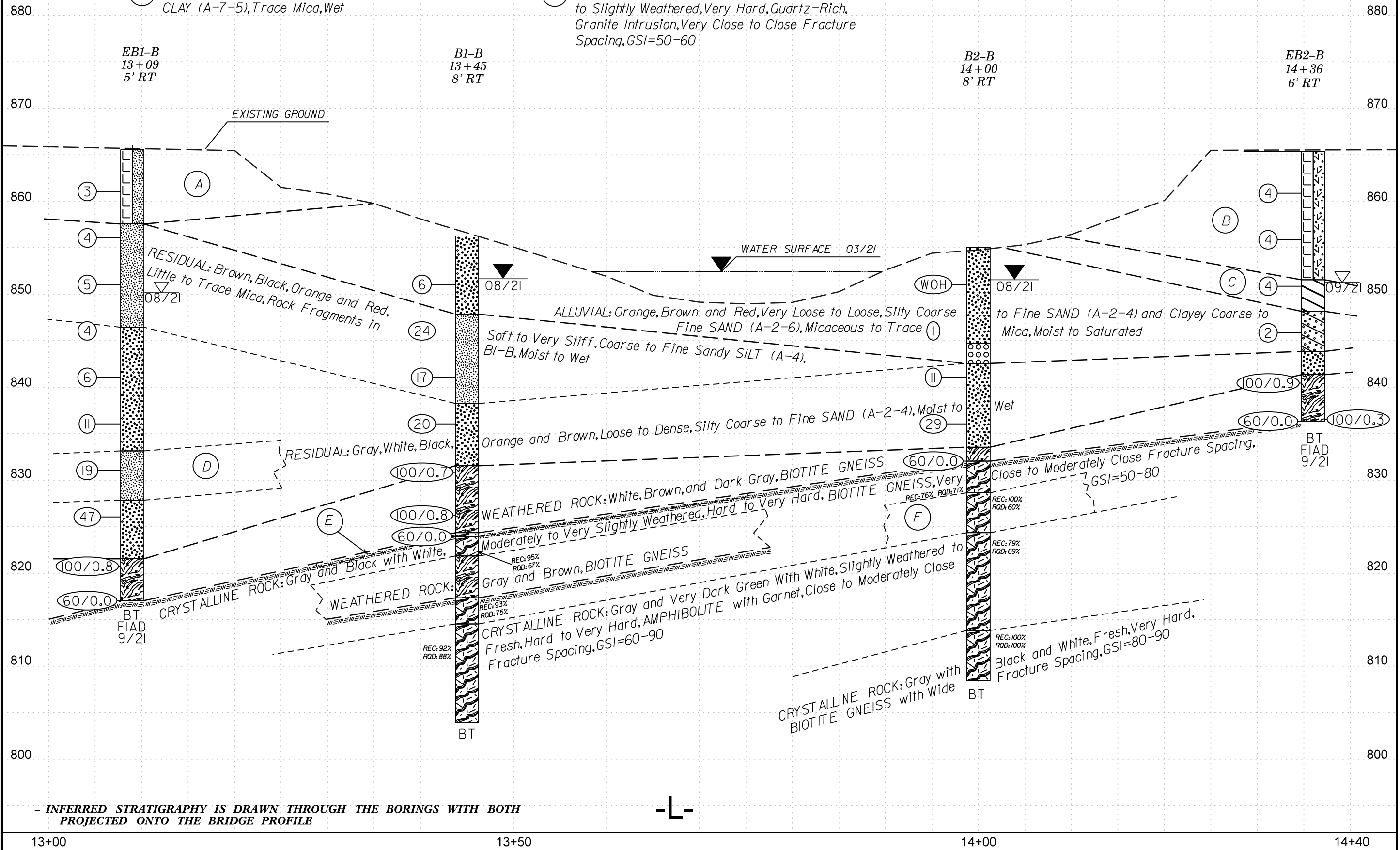
ROADWAY EMBANKMENT: Red, Orange and Brown, Soft to Medium Stiff, Clayey SILT (A-5), Trace Mica, Moist to Wet
- C

ALLUVIAL: Brown, Soft to Medium Stiff, Silty CLAY (A-7-5), Trace Mica, Wet
- D

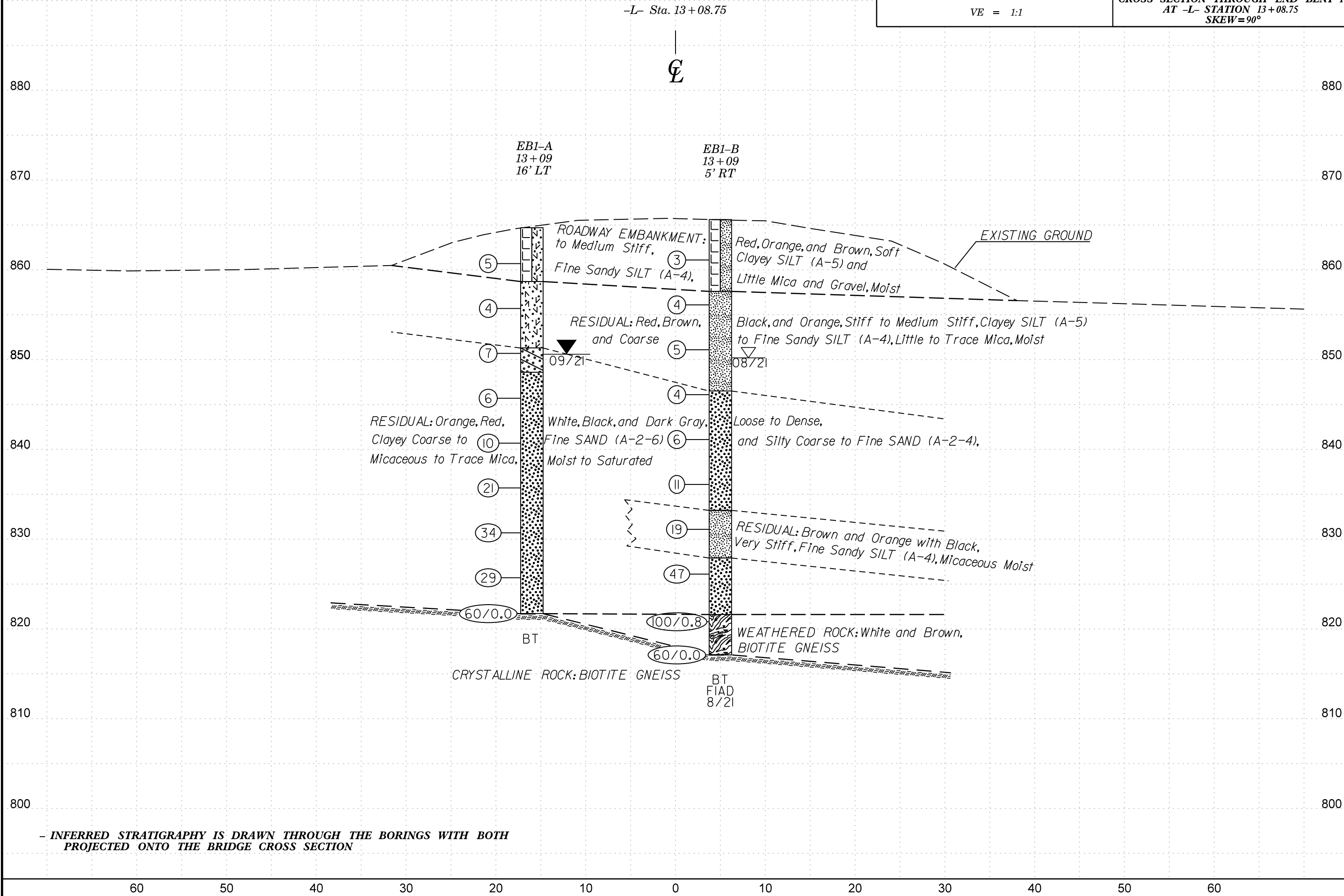
RESIDUAL: Brown and Orange with Black, Very Stiff, Fine Sandy SILT (A-4), Micaceous, Moist
- E

CRYSTALLINE ROCK: White and Dark Red, Moderately Severely Weathered, BIOTITE GNEISS
- F

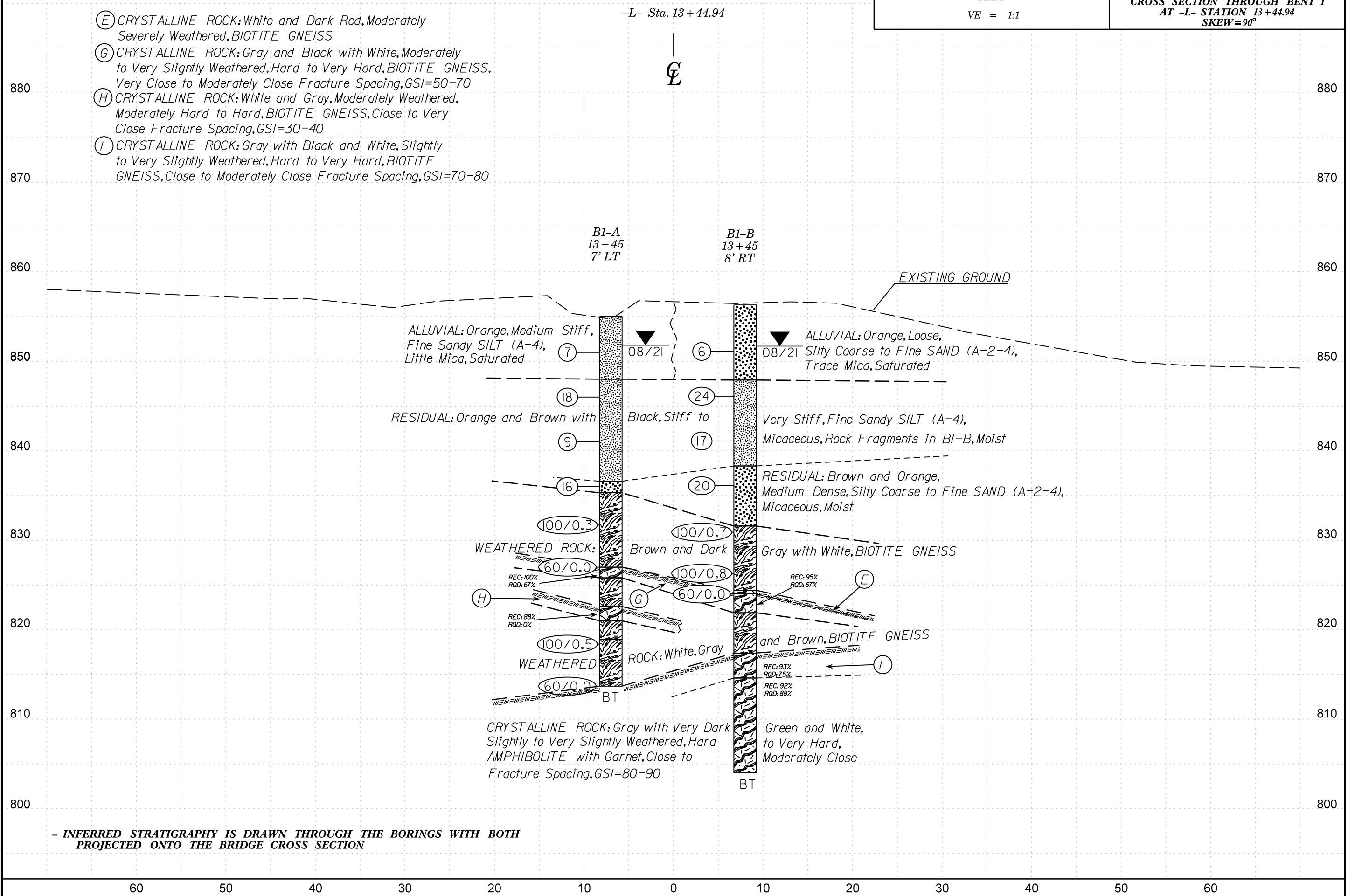
CRYSTALLINE ROCK: White with Gray, Moderately to Slightly Weathered, Very Hard, Quartz-Rich, Granite Intrusion, Very Close to Close Fracture Spacing, GSI=50-60

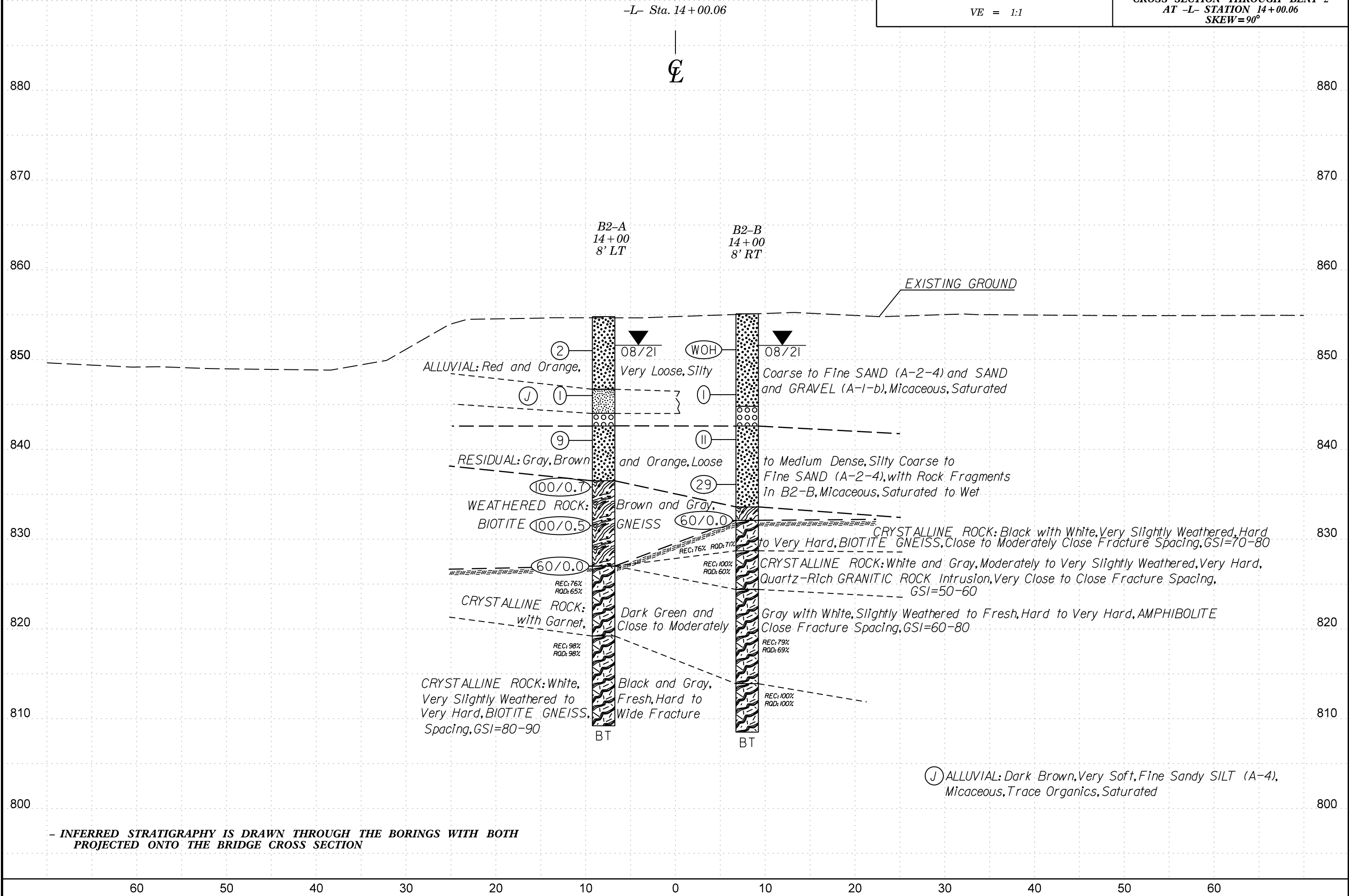


- INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE BRIDGE PROFILE -L-







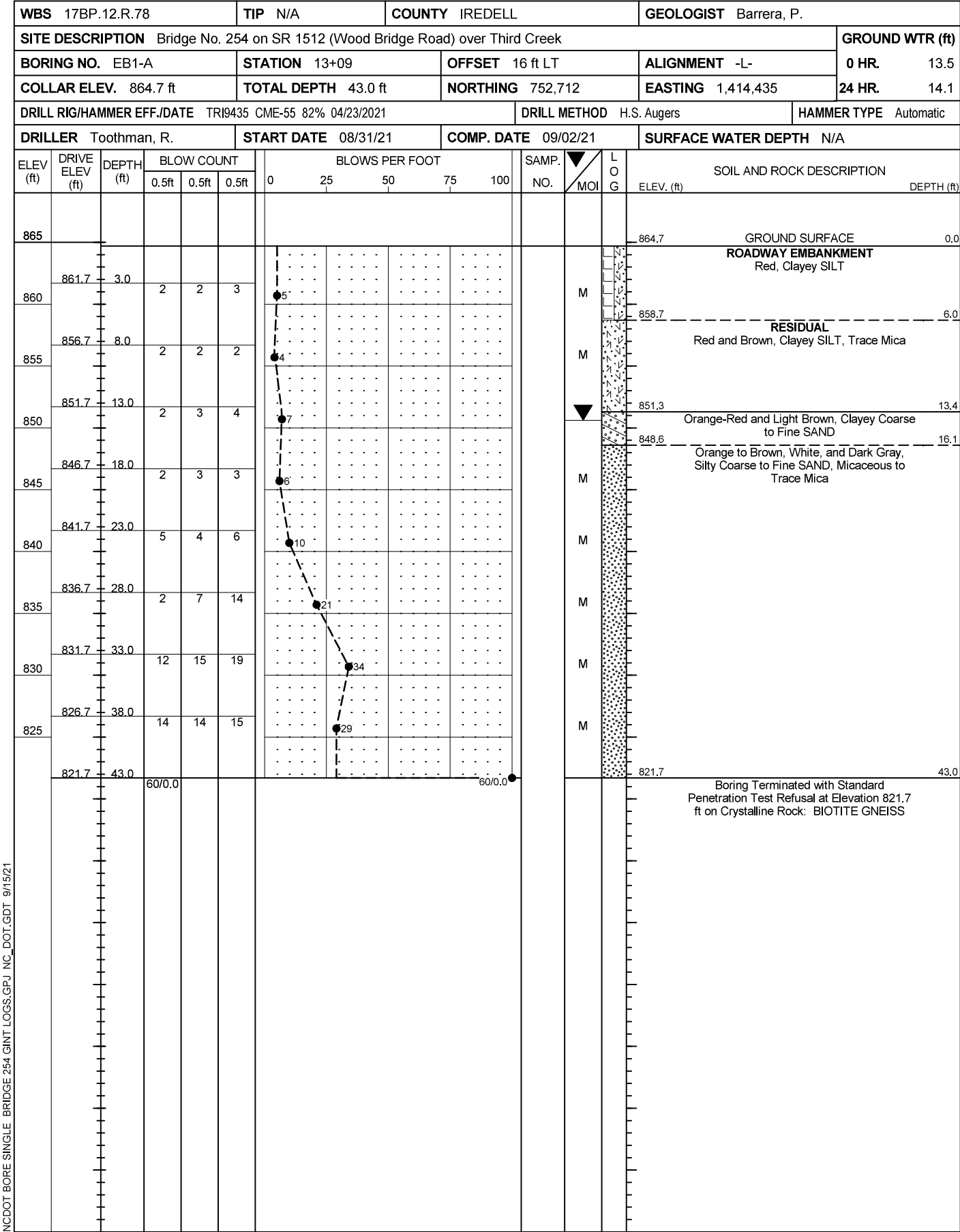






GEOTECHNICAL BORING REPORT  
BORE LOG

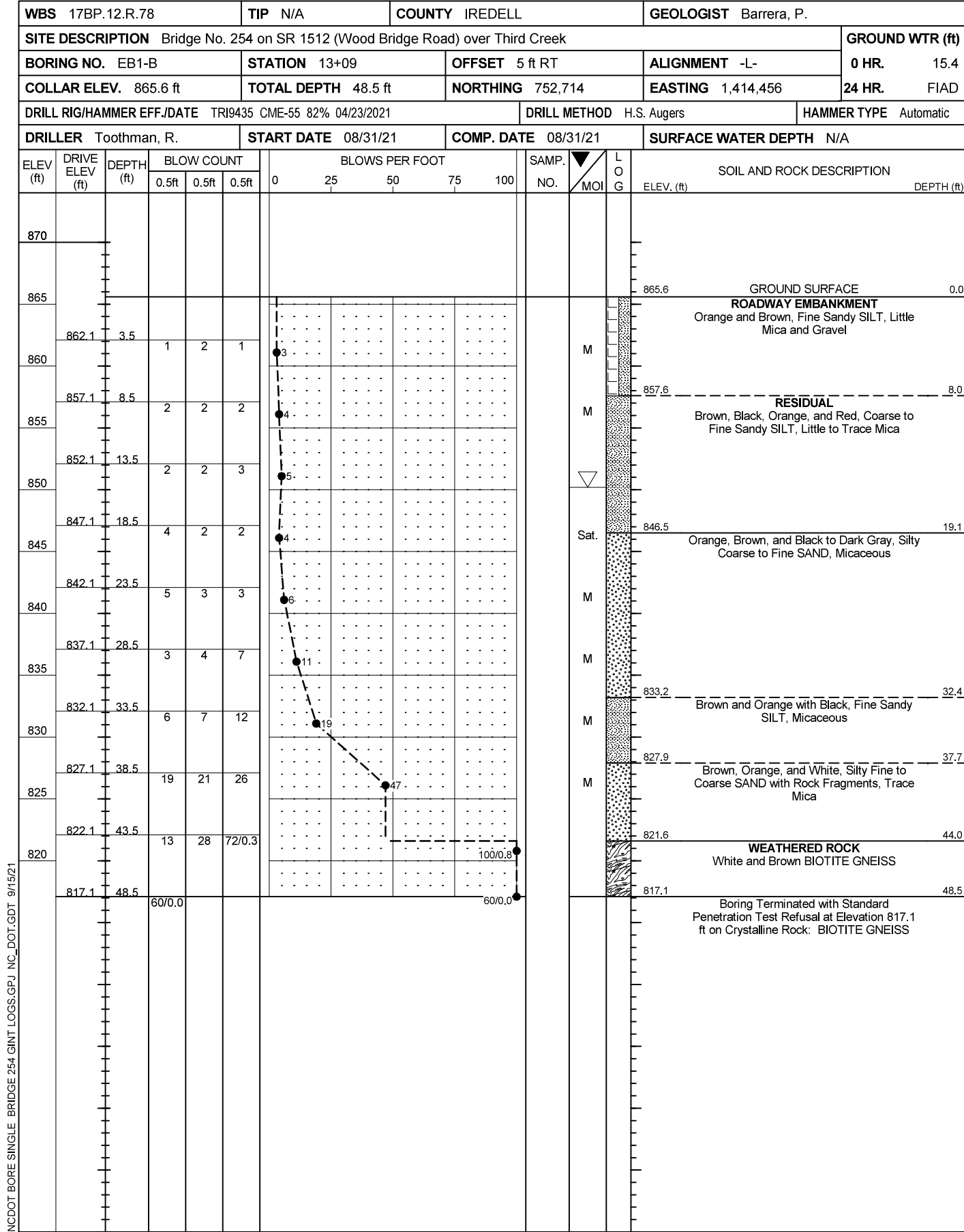
SHEET 9



NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

GEOTECHNICAL BORING REPORT  
BORE LOG

SHEET 9



NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

GEOTECHNICAL BORING REPORT  
BORE LOG

SHEET 10

WBS 17BP.12.R.78				TIP N/A				COUNTY IREDELL				GEOLOGIST Barrera, P.					
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek												GROUND WTR (ft)					
BORING NO. B1-A				STATION 13+45				OFFSET 7 ft LT				ALIGNMENT -L-				0 HR. 3.1	
COLLAR ELEV. 855.0 ft				TOTAL DEPTH 41.3 ft				NORTHING 752,749				EASTING 1,414,440				24 HR. 3.2	
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021								DRILL METHOD SPT Core Boring				HAMMER TYPE Automatic					
DRILLER Toothman, R.				START DATE 08/26/21				COMP. DATE 08/27/21				SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)		
855														855.0 GROUND SURFACE 0.0			
850	852.0	3.0	4	3	4								▼	ALLUVIAL Orange, Fine Sandy SILT, Little Mica			
845	847.0	8.0	8	7	11								M	848.0 7.0 RESIDUAL Orange, Brown, and Black, Fine Sandy SILT, Micaceous			
840	842.0	13.0	2	4	5								M				
835	837.0	18.0	4	7	9								M	836.6 18.4 Brown and Orange, Silty Coarse to Fine SAND with Rock Fragments, Micaceous			
830	832.0	23.0	100/0.3											835.3 19.7 WEATHERED ROCK Brown and White, BIOTITE GNEISS			
825	827.0	28.0	60/0.0											827.0 28.0 CRYSTALLINE ROCK Gray and Black with White, Very Slightly Weathered, Hard, BIOTITE GNEISS, Moderately Close Fracture Spacing			
820														822.6 32.4 WEATHERED ROCK Brown with White and Red, BIOTITE GNEISS			
	818.9	36.1	100/0.5														
815														821.0 34.0 CRYSTALLINE ROCK White and Gray, Moderately Weathered, Moderately Hard to Hard, BIOTITE GNEISS, Close to Very Close Fracture Spacing			
	813.7	41.3	60/0.0														
														818.9 36.1 WEATHERED ROCK White and Gray with Brown, BIOTITE GNEISS			
														813.7 41.3 WEATHERED ROCK White and Gray with Brown, BIOTITE GNEISS			
														Boring Terminated at Elevation 813.7 ft on Crystalline Rock: BIOTITE GNEISS			
														Sand flowing into borehole during coring. Casing could not be extended past top of weathered rock. Flowing sand prevented coring past 36.1'.			

NCDOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

GEOTECHNICAL BORING REPORT  
CORE LOG

SHEET 10

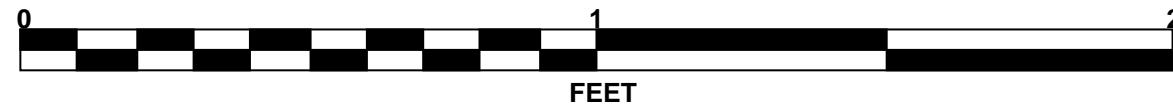
WBS 17BP.12.R.78				TIP N/A				COUNTY IREDELL				GEOLOGIST Barrera, P.							
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek												GROUND WTR (ft)							
BORING NO. B1-A				STATION 13+45				OFFSET 7 ft LT				ALIGNMENT -L-				0 HR. 3.1			
COLLAR ELEV. 855.0 ft				TOTAL DEPTH 41.3 ft				NORTHING 752,749				EASTING 1,414,440				24 HR. 3.2			
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021								DRILL METHOD SPT Core Boring				HAMMER TYPE Automatic							
DRILLER Toothman, R.				START DATE 08/26/21				COMP. DATE 08/27/21				SURFACE WATER DEPTH N/A							
CORE SIZE NQ				TOTAL RUN 8.1 ft															
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %		RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %		RQD (ft) %	L O G	DESCRIPTION AND REMARKS				DEPTH (ft)		
827													Begin Coring @ 28.0 ft						
825	827.0	28.0	3.1	0:18/0.1 2:16/1.0 1:53/1.0 3:55/1.0	(1.5) 48%	(0.8) 26%			(1.2) 100%	(0.8) 67%			827.0 825.8	CRYSTALLINE ROCK Gray and Black with White, Very Slightly Weathered, Hard, BIOTITE GNEISS, Moderately Close Fracture Spacing 1 joint at 90 degrees in the first 0.6' GSI=60-70				28.0 29.2	
	823.9	31.1	5.0	1:33/1.0 2:42/1.0 1:04/1.0 0:55/1.0 1:53/1.0	(1.4) 28%	(0.0) 0%			(0.3) 9%	(0.0) 0%			822.6					32.4	
820	818.9	36.1							(1.4) 88%	(0.0) 0%			821.0	WEATHERED ROCK Brown with White and Red, BIOTITE GNEISS				34.0	
										(0.0) 0%	(0.0) 0%			818.9					36.1
815				N=100/0.5										CRYSTALLINE ROCK White and Gray, Moderately Weathered, Moderately Hard to Hard, BIOTITE GNEISS, Close to Very Close Fracture Spacing 2 joints at 90 degrees with iron staining, remaining joints at 0 degrees to 10 degrees GSI=30-40					
				N=60/0.0										813.7	WEATHERED ROCK White and Gray with Brown, BIOTITE GNEISS Boring Terminated at Elevation 813.7 ft on Crystalline Rock: BIOTITE GNEISS				41.3
Sand flowing into borehole during coring. Casing could not be extended past top of weathered rock. Flowing sand prevented coring past 36.1'.																			

NC DOT CORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

## CORE PHOTOGRAPHS

**B1-A**

BOX 1: 28.0 - 36.1 FEET



## SHEET 12

NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC DOT.GDT 9/15/21

## SHEET 12

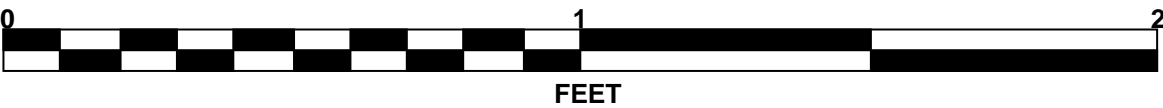
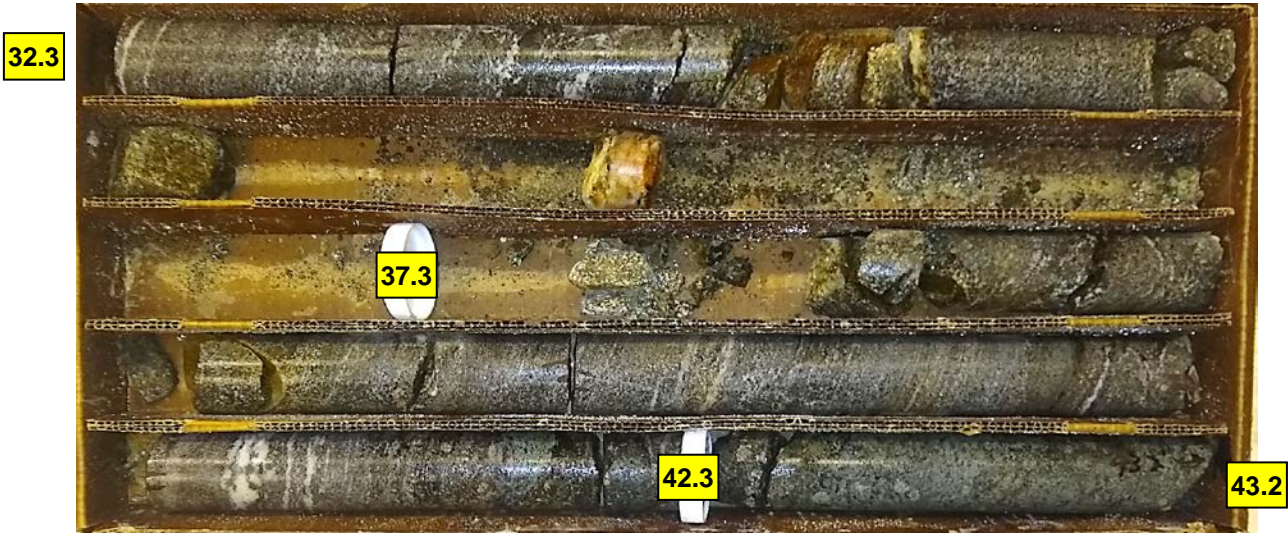
NCDOT CORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC DOT.GDT 9/15/21



CORE PHOTOGRAPHS

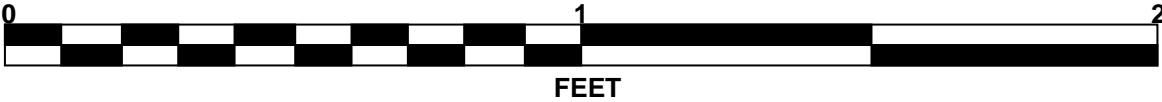
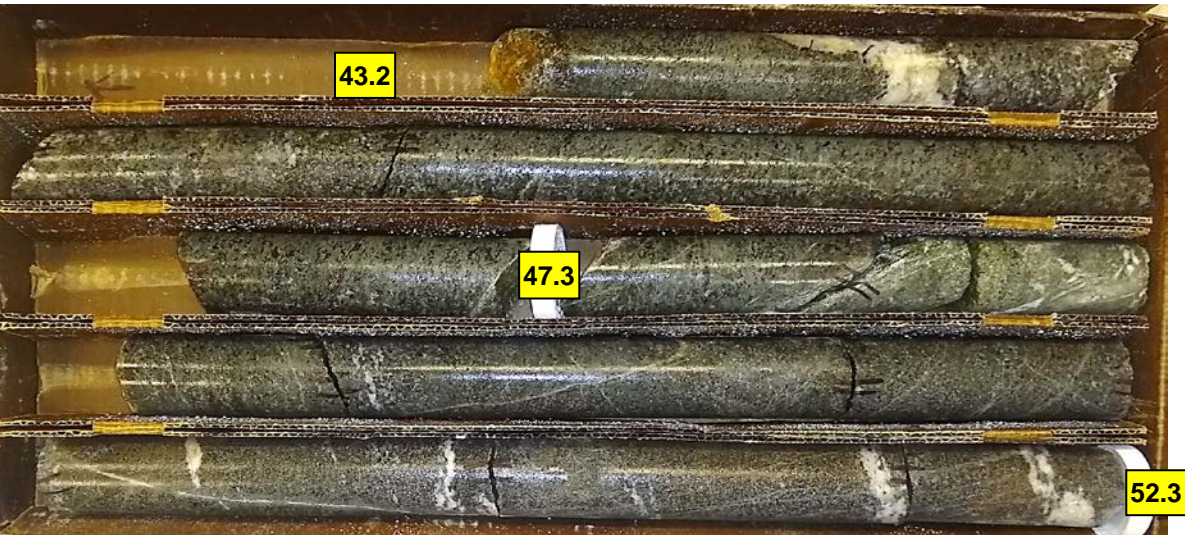
B1-B

BOX 1: 32.3 - 43.2 FEET



B1-B

BOX 2: 43.2 - 52.3 FEET



GEOTECHNICAL BORING REPORT  
 BORE LOG

SHEET 14

WBS 17BP.12.R.78				TIP N/A				COUNTY IREDELL				GEOLOGIST Barrera, P.					
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek												GROUND WTR (ft)					
BORING NO. B2-A				STATION 14+00				OFFSET 8 ft LT				ALIGNMENT -L-				0 HR. N/A	
COLLAR ELEV. 854.8 ft				TOTAL DEPTH 45.6 ft				NORTHING 752,804				EASTING 1,414,434				24 HR. 3.2	
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021								DRILL METHOD SPT Core Boring				HAMMER TYPE Automatic					
DRILLER Toothman, R.				START DATE 08/23/21				COMP. DATE 08/24/21				SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)			
855														854.8	GROUND SURFACE	0.0	
850	852.0	2.8	3	1	1	2							▼	Sat.	ALLUVIAL Red, Silty Coarse to Fine SAND, Micaceous		
	847.0	7.8	2	WOH	1	1								Sat.	846.7	8.1	
840	842.0	12.8	12	5	4	9								Sat.	844.0 842.6	10.8 12.2	
	837.0	17.8	13	28	72/0.2										RESIDUAL Gray and Brown, Silty Coarse to Fine SAND, Micaceous		
835											100/0.7			836.5	WEATHERED ROCK Gray and Brown, BIOTITE GNEISS	18.3	
830	832.0	22.8	100/0.5								100/0.5						
	827.0	27.8	60/0.0								60/0.0			827.0	CRYSTALLINE ROCK Very Dark Green and Gray with White, Slightly to Very Slightly Weathered, Hard, AMPHIBOLITE, with Garnet, Close Fracture Spacing	27.8	
820														819.2	White, Black, and Gray, Very Slightly Weathered to Fresh, Hard to Very Hard, BIOTITE GNEISS, Wide Fracture Spacing	35.6	
815																	
810														809.2	Boring Terminated at Elevation 809.2 ft in Crystalline Rock: BIOTITE GNEISS	45.6	

NCDOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

GEOTECHNICAL BORING REPORT  
 CORE LOG

SHEET 14

WBS 17BP.12.R.78					TIP N/A			COUNTY IREDELL			GEOLOGIST Barrera, P.				
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek										GROUND WTR (ft)					
BORING NO. B2-A					STATION 14+00			OFFSET 8 ft LT			ALIGNMENT -L-			0 HR. N/A	
COLLAR ELEV. 854.8 ft					TOTAL DEPTH 45.6 ft			NORTHING 752,804			EASTING 1,414,434			24 HR. 3.2	
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021								DRILL METHOD SPT Core Boring				HAMMER TYPE Automatic			
DRILLER Toothman, R.					START DATE 08/23/21			COMP. DATE 08/24/21			SURFACE WATER DEPTH N/A				
CORE SIZE NQ					TOTAL RUN 17.8 ft										
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %		RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %		RQD (ft) %	L O G	DESCRIPTION AND REMARKS		DEPTH (ft)
827													Begin Coring @ 27.8 ft		
825	827.0	27.8	2.8	3:28/0.8 3:18/1.0 2:37/1.0	(2.1) 75%	(1.4) 50%			(5.9) 76%	(5.1) 65%			827.0	27.8	CRYSTALLINE ROCK Very Dark Green and Gray with White, Slightly to Very Slightly Weathered, Hard, AMPHIBOLITE, with Garnet, Close Fracture Spacing Joints at 10 degrees to 20 degrees GSI=70-80
	824.2	30.6	5.0	2:14/1.0 1:59/1.0 2:16/1.0 3:16/1.0 3:58/1.0	(3.8) 76%	(3.7) 74%									
820	819.2	35.6											819.2	35.6	
			5.0	3:13/1.0 2:59/1.0 3:30/1.0 3:27/1.0 3:52/1.0	(4.8) 96%	(4.8) 96%		(9.8) 98%	(9.8) 98%				White, Black, and Gray, Very Slightly Weathered to Fresh, Hard to Very Hard, BIOTITE GNEISS, Wide Fracture Spacing No natural fractures GSI=80-90		
815	814.2	40.6													
			5.0	2:53/1.0 3:25/1.0 3:09/1.0 3:16/1.0 3:15/1.0	(5.0) 100%	(5.0) 100%									
810	809.2	45.6											809.2	45.6	
													Boring Terminated at Elevation 809.2 ft in Crystalline Rock: BIOTITE GNEISS		

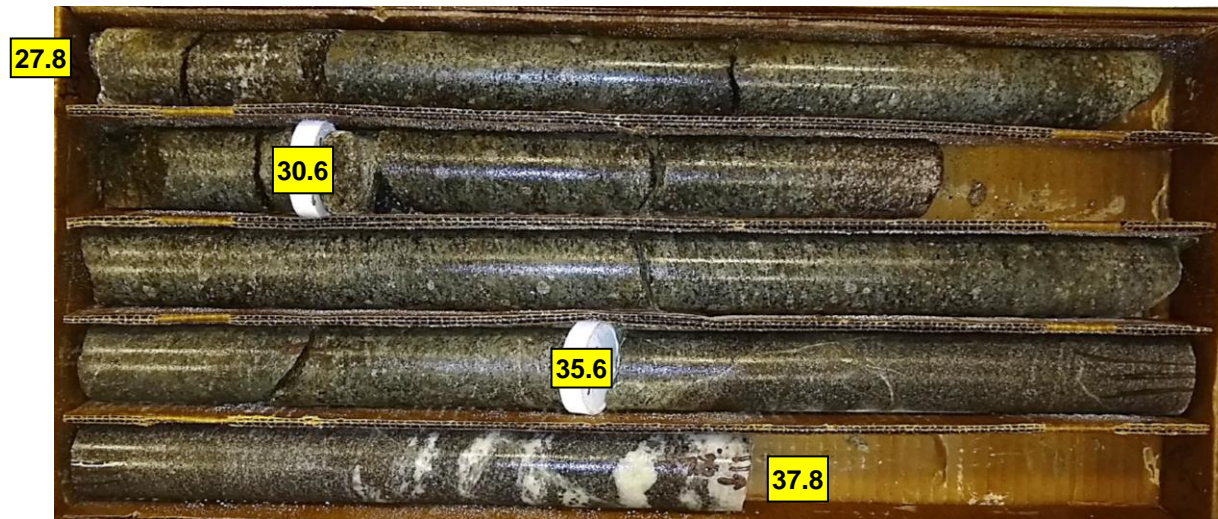
NCDOT CORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21



## CORE PHOTOGRAPHS

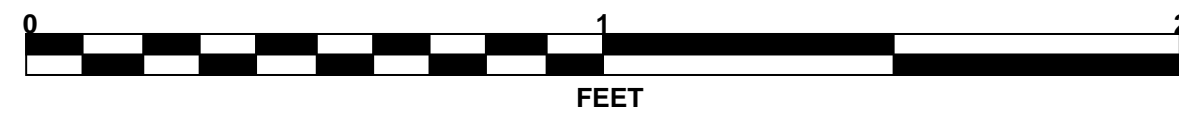
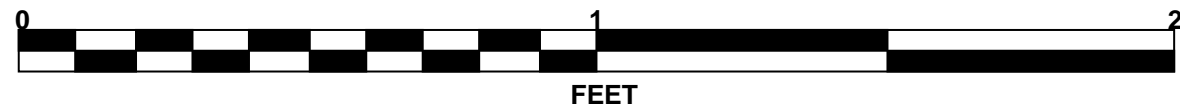
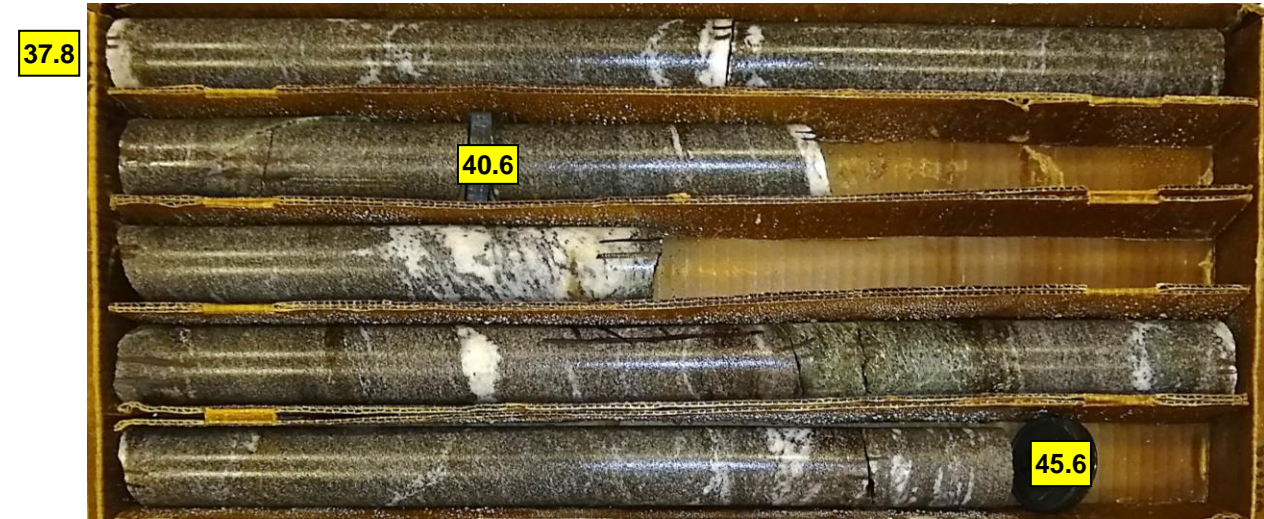
### B2-A

BOX 1: 27.8 - 37.8 FEET



### B2-A

BOX 2: 37.8 - 45.6 FEET



GEOTECHNICAL BORING REPORT  
BORE LOG





SHEET 16

WBS 17BP.12.R.78		TIP N/A		COUNTY IREDELL		GEOLOGIST Barrera, P.						
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek							GROUND WTR (ft)					
BORING NO. B2-B		STATION 14+00		OFFSET 8 ft RT		ALIGNMENT -L-						
COLLAR ELEV. 855.1 ft		TOTAL DEPTH 46.6 ft		NORTHING 752,805		EASTING 1,414,449						
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021				DRILL METHOD SPT Core Boring		HAMMER TYPE Automatic						
DRILLER Toothman, R.		START DATE 08/24/21		COMP. DATE 08/25/21		SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT		SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100	ELEV. (ft)	DEPTH (ft)
860												
855											855.1	0.0
850	852.1	3.0	WOH	WOH	WOH	0.					GROUND SURFACE	
845	847.1	8.0	2	WOH	1	1.					ALLUVIAL	
840	842.1	13.0	6	5	6						Orange, Silty Fine to Coarse SAND, Micaceous	
835	837.1	18.0	16	16	13							
830	832.1	23.0	60/0.0								SAND and Gravel (Old Stream Bed Material)	
825											RESIDUAL	
820											Gray, Brown, and Orange, Silty Coarse to Fine SAND with Rock Fragments, Micaceous	
815												
810											WEATHERED ROCK	
											Brown and Gray, BIOTITE GNEISS	
											CRYSTALLINE ROCK	
											Black with White, Very Slightly Weathered, Hard to Very Hard, BIOTITE GNEISS, Close to Moderately Close Fracture Spacing	
											White with Gray, Moderately to Very Slightly Weathered, Very Hard, Quartz-Rich GRANITIC ROCK Intrusion, Very Close to Close Fracture Spacing	
											Very Dark Green with White, Very Slightly Weathered to Fresh, Hard to Very Hard, AMPHIBOLITE with Garnet, Close to Moderately Close Fracture Spacing	

NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

GEOTECHNICAL BORING REPORT  
CORE LOG

SHEET 16

WBS 17BP.12.R.78				TIP N/A		COUNTY IREDELL				GEOLOGIST Barrera, P.				
SITE DESCRIPTION Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek										GROUND WTR (ft)				
BORING NO. B2-B				STATION 14+00				OFFSET 8 ft RT				ALIGNMENT -L-		0 HR. N/A
COLLAR ELEV. 855.1 ft				TOTAL DEPTH 46.6 ft				NORTHING 752,805				EASTING 1,414,449		24 HR. 3.5
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 82% 04/23/2021								DRILL METHOD SPT Core Boring				HAMMER TYPE Automatic		
DRILLER Toothman, R.				START DATE 08/24/21				COMP. DATE 08/25/21				SURFACE WATER DEPTH N/A		
CORE SIZE NQ					TOTAL RUN 23.6 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) % RQD (ft) %		SAMP. NO.	STRATA REC. (ft) % RQD (ft) %		L O G	DESCRIPTION AND REMARKS			DEPTH (ft)
832.1														
830	832.1	23.0	3.6	0:54/0.6 2:07/1.0 2:41/1.0 2:55/1.0	(2.8) 78%	(2.3) 64%		(2.6) 76%	(2.4) 71%		832.1	Begin Coring @ 23.0 ft		23.0
	828.5	26.6		2:49/1.0 3:32/1.0 4:02/1.0 5:28/1.0 3:20/1.0	(5.0) 100%	(3.6) 72%		(4.3) 100%	(2.6) 60%		828.7	CRYSTALLINE ROCK Black with White, Very Slightly Weathered, Hard to Very Hard, BIOTITE GNEISS, Close to Moderately Close Fracture Spacing Joints at 10 degrees to 20 degrees GSI=70-80		26.4
825			5.0	2:37/1.0 3:40/1.0 3:09/1.0 2:21/1.0 3:14/1.0	(2.8) 56%	(2.4) 48%		(8.3) 79%	(7.2) 69%		824.4	White with Gray, Moderately to Very Slightly Weathered, Very Hard, Quartz-Rich GRANITIC ROCK Intrusion, Very Close to Close Fracture Spacing Note: AMPHIBOLITE comprises half of core in last 0.9' Majority of joints at 10 degrees to 20 degrees One near vertical joint in last 0.9' GSI=50-60		30.7
	823.5	31.6		3:33/1.0 3:15/1.0 3:00/1.0 3:43/1.0 4:21/1.0	(5.0) 100%	(4.3) 86%						Very Dark Green and White, Very Slightly Weathered to Fresh, Hard to Very Hard, AMPHIBOLITE with Garnet, Close to Moderately Close Fracture Spacing Joints at 10 degrees to 20 degrees GSI=60-70		
820	818.5	36.6		3:58/1.0 5:09/1.0 2:40/1.0 2:49/1.0 3:02/1.0	(5.0) 100%	(5.0) 100%		(5.4) 100%	(5.4) 100%		813.9	Gray with Black and White, Fresh, Very Hard, BIOTITE GNEISS, Wide Fracture Spacing No natural fractures GSI=80-90		41.2
	813.5	41.6												
810			5.0								808.5	Boring Terminated at Elevation 808.5 ft in Crystalline Rock: BIOTITE GNEISS		46.6
	808.5	46.6												

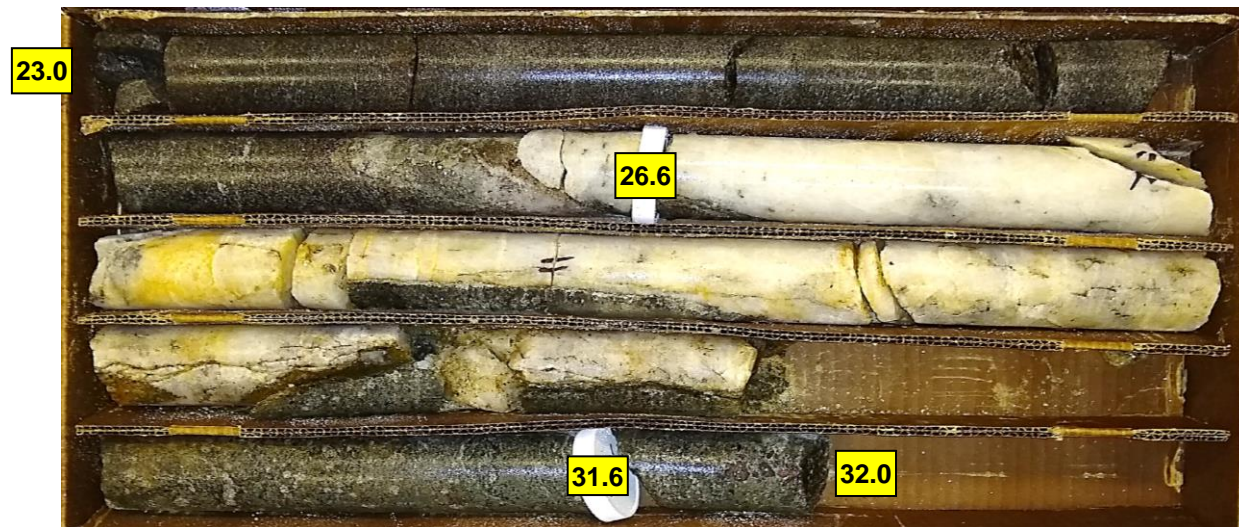
NC DOT CORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21



## CORE PHOTOGRAPHS

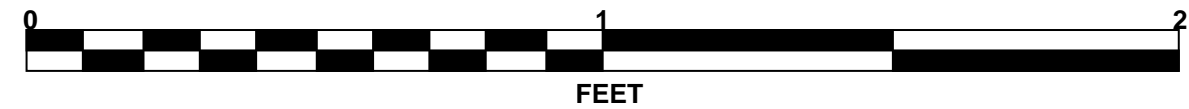
### B2-B

BOXES 1 & 2: 23.0 - 41.6 FEET



### B2-B

BOX 3: 41.6 - 46.6 FEET



SHEET 18

NC DOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21

## SHEET 18

NCDOT BORE SINGLE BRIDGE 254 GINT LOGS.GPJ NC\_DOT.GDT 9/15/21



**SITE PHOTOGRAPHS**  
Bridge No. 254 on SR 1512 (Wood Bridge Road) over Third Creek

View of Bridge 254 Looking Upstation



View Looking Upstream from Bridge 254



View of Bridge 254 Looking Downstation



View Looking Downstream from Bridge 254

